35 Otselic River at S.R. 23 at Cincinnatus, NY

Site Location:								
Site ID:	35							
Site Name:	Otselic River at S.R. 23 at Cincinnatus, NY							
County:	Cortland							
Nearest City:	Cincinnatus	Contact:						
State:	NY	Pete Riehlman, NYSDOT, Region 3 hydraulic engineer (315) 428-4715.						
Latitude:	423200							
Longitude:	755412							
USGS Station ID:	1510200							
Route Number:	23							
Route Class:	State	Publication:						
Service Level:	Mainline							
Route Direction:	NA							
Highway Mile Poir	nt:							
Stream Name:	Otselic River							
River Mile:								

Site Description:

This site is located at the State Route 23 bridge crossing the Otselic River at Cincinnatus, New York. The bridge, 187 ft long and 40 ft wide with one pier, is 0.5 mile downstream from a USGS streamflow gage. The pier footing has been undermined by scour, but the bridge is supported by pilings. A 30-degree angle between the pier and flow increases the tendency of the streambed to scour and has resulted in scour along the entire length of the pier. The deepest scour is 10 to 25 ft downstream from the pier nose.

Clear-water scour is common at this site. Multiple high flows have progressively deepened the local-scour hole. Each additional scour event is being analyzed separately for the New York study. However for the USGS national scour study, the "total" local scour is the depth of scour that the earlier high flows may have produced if the flow duration was sufficient to produce an equilibrium scour depth. The local-scour value listed in this data base for the 1990 high flow includes the previous local scour from earlier high flows.

General or contraction scour may have lowered the ambient bed 1-2 ft from the 1027 ft elevation listed in the bridge plans. The USGS stage-discharge relation can account for 0.3 ft. Contraction scour is insignificant based on approach, bridge, and exit cross sections.

The streambed is armored by gravel. Occassional mining in the stream about 0.5 mile upstream from the bridge at point bars does not appear to be degrading the thalweg. However, the thalweg may be migrating into the local-scour hole because the hole is now the lowest point in the channel.

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Bed-material samples were collected in a shallow area of the channel near the bridge. The D16, D50, and D84 were analyzed. The D90 or D95 were not analyzed because of the accuracy of the limited data set.

Debris and high flow in 1993 lowered the channel near station 120 to 1019 ft msl, but the debris prevented additional local scour at the nose of the pier.

The local-scour hole does not refill after each high flow.

Local-scour depth is based on USGS measurements, although New York State Department of Transportation (NYSDOT) measurements are considered in the analysis. The 1988 NYSDOT ambient bed is questionable because it is 1.3 ft higher than the 1989 USGS measurement, and no significant high flow occurred. A USGS measurement in 1989 found scour hole at 1022.7 ft at the upstream side of the bridge (ussb). Because ambient bed is 1025.5 ft, 2.8 ft of local scour is calculated. The location of maximum scour was not measured in 1989, based on later data that show the deepest scour located 10-25 ft downstream from the pier nose. High flows in 1983, 1984, and 1986 are assumed to have contributed to the scour measured in 1989.

High flow in 1990 lowered the scour hole to 1020.6 at ussb and 1019.9 25 ft downstream. The ambient bed was lowered 0.4 ft, therefore, local scour is calculated to be 5.2 ft under the bridge (1025.1-1019.9).

In the New York study, each high flow is analyzed separately. Because the elevation of the scour hole under the bridge is unknown before the 1990 high flow, 1.7 ft of local scour is attributed to the 1990 high flow based on ussb data (scour hole was 2.1 ft deeper and ambient bed was 0.4 ft lower in 1991 than in 1990).

Elevation Reference

Datum: MSL

MSL (ft):

Description of Reference Elevation:

Stream Data

Drainage Area (sq mi):	153	Floodplain Width:	Narrow
Slope in Vicinity(ft/ft):	0.0004	Natural Levees:	Concave
Flow Impact:	Straight	Apparent Incision:	None

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Channel Evolution	Premodified	Channel Boundary:	Alluvial
Armoring:	High	Banks Tree Cover:	Low
Debris Frequency:	Occasional	Sinuosity:	Sinuous
Debris Effect:	Local	Braiding:	None
Stream Size:	Medium	Anabranching:	None
Flow Habit:	Flashy	Bars:	Narrow
Bed Material:	Gravel	Stream Width Variability:	Unknown
Valley Setting:	Moderate		

Roughness Data

Manning's n Values

	Left Overbank	Channel	Right Overbank
High:			
Typical	0.05	0.033	0.07
Low:			

Bed Material

Measurement Number	Yr	Мо	Dy	Sampler	D95 (mm)	D84 (mm)	D50 (mm)	D16 (mm)	SP	Shape	Cohesion
1	1989	10	24	GRID	76	55	32	18	2.65		Non-Cohesive
2	1989	10	24	SHOVEL		33	15	2	2.65		Non-Cohesive

Bed Material Comments

Measurement No: 1

Bed-material samples were collected in a shallow area of the channel near the bridge. The sizes are based on 100 samples using the grid-sampling technique

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Measurement No: 2

Bed-material samples were collected in a shallow area of the channel near the bridge. The D16, D50, and D84 were analyzed. The D90 or D95 were not analyzed because of the accuracy of the limited data set.

Bridge Data

Bridge Bula	
Structure No:	3312170
Length(ft):	187
Width(ft):	40
Number of Spans:	2
Vertical Configur	ation: Horizontal
Low Chord Elev (f	t): 1036
Upper Chord Elev	(ft):
Overtopping Elev	(ft): 1042
Skew (degrees):	30
Guide Banks:	None
Waterway Classifi	cation: Main
Year Built:	1981
Avg Daily Traffic	:
Plans on File:	Yes
Parallel Bridges	No
Upstream/Downstre	am: N/A
Continuous Abutme	nt: No
Distance Between	Centerlines:
Distance Between	Pier Faces:
Bridge Descript:	ion:

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Abutment Data

Left Station:	0		
Right Station:	186		
Left Skew (deg):	0		
Right Skew (deg)	0		
Left Abutment Len	gth (ft):		
Right Abutment Le	ngth (ft)		
Left Abutment to	Channel Ba	nk (ft):	20
Right Abutment to	Channel B	ank (ft):	50
Left Abutment Pro	tection:		
Right Abutment Pr	otection		
Contracted Openin	g Type:	III	
Embankment Skew (deg):	0	
Embankment Slope	(ft/ft):		
Abutment Slope (f	t/ft)		
Wingwalls:		Yes	
Wingwall Angle (d	eg):	0	

Pier Data

Pier ID	Bridge Station(ft)	Alignment	Highway	Station	n PierType	# Of Piles	Pile Spacing(ft)					
1	96	0	9	6	Single	0						
Pier ID	Pier Width(ft)	Pier Shape	Shape 1	Factor	Length(ft)	Protection	Foundation					
1	3	Round			40	None	Piles					
Pier ID	Top Elevation(Bo ft) Eleva	ottom ation(ft)	Foot Cap W	or Pile Width(ft)	Cap Shape	Pile Tip Elevation(ft)					
1	1025.5		1022		6	Square						
Pier De	Pier Description											

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Pier ID

1

The flow angle of 30 degrees results in scour along the entire length of the right side of the pier. The footing is undermined 2 ft. Flow aligns with the pier before reaching the downstream side.

Pier Scour Data

Pier	ID	D	ate	1	ſime	USOrDS					
1		10/	24/90	(00:00	Upstream	L				
Pier ID	Sc De	our pth	Accur (ft	acy S :)	ide Slope (ft/ft)	TopWidth (ft)	Appro Vel (ft	h Ap :/s)Dep	pprch pth(ft)	Effective Pier Width	Skew to Flow(deg)
1		5.2	0.2	2	6.6	52	6.8		10.3	3	30
PierII	D	Sedim Trans	ent port	1 Mat	Bed erial	BedForm	Trough (ft)	Crest (ft)	Sigma	Debris Effects	l .
1	С	lear-	water	Non-	cohesive	Unknown			1.7	5 Insignif	icant
Pie	erI	D	D95	(mm)	D84 (mm) D50	(mm)	D16	(mm)		
	1		7	6	55		32		18		

Pier Scour Comments

Pier	ID	1	Time: 0:00	US/DS:	Upstream
					- <u> </u>

The sounding weight became lodged under the footing during measurement. Local scour at the upstream side of the bridge (ussb) is 4.5 ft. The maximum local scour is 5.2 ft 25 ft downstream from ussb, based on the 8-28-91 cross section. The ambient bed was lowered 0.4 ft.

Abutment Scour

ContractionScour

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Stage and Discharge Data

Pea	ak D	isch	arge	•	Flow		Peak Stage			Stage	Water	Return		
year	mo	dy	hr	mi	(cfs)	Qacc	year	mo	dy	hr	mi	(ft)	Temp (C)	Period(yr)
1993	4	11		0	7300	95	1993	4	11		0			15
1983	12	14		0	6600	95	1983	12	14		0			8
1984	2	15		0	5900	95	1984	2	15		0			5
1986	3	15		0	6200	95	1986	3	15		0			6
1990	10	24		0	6100	95	1990	10	24		0			6

Hydrograph

Hydrograph								Discharge
Number	Year	Month	Day	Hr	Min	Sec	Stage(ft)	(cfs)

Supporting Files