1 Susitna River at S.R. 3 near Sunshine, AK

Site Location:		
Site ID:	1	
Site Name:	Susitna River at S.R. 3 near Suns	shine, AK
County:	Town of Sunshine	
Nearest City:	Sunshine	Contact: U.S. Geological Survey, Water
State:	AK	Resources Division 218 E Street, Skyline Building
Latitude:	621000	Anchorage, AK 99501
Longitude:	1500500	
USGS Station ID:		
Route Number:	3	
Route Class:	State	Publication: U.S. Geological Survey
Service Level:	Mainline	Water-Resources Investigations 32- 75
Route Direction:	NA	Scour at Selected Bridge Sites in Alaska
Highway Mile Poir		By Vernon W. Norman November 1975
Stream Name:	Susitna River	
River Mile:		

Site Description:

This study site is located on the Susitna River at mile 104 of the Anchorage-Fairbanks Highway, about 3.2 miles west of Sunshine and 10.5 miles downstream from the mouth of the Talkeetna River. The Susitna River basin above the bridge site covers about 11,500 square miles. Less than 15 percent of the area is occupied by glaciers. In the vicinity of the bridge site the river channel is braided and consists of multiple bars and islands. Surface bed material consists of gravel and cobbles and some sand. Floodflows on the Susitna result from snow melt in the spring and from rainfall combined with glacial melt water in mid to late summer. Records of flood data have been collected about 50 miles upstream on the Susitna River at Gold Creek since 1949. The peak flow occurred June 7, 1964. In 14 of these years, floods occurred in June. The mean annual flood for the scour site was estimated to be about 80,000 cfs. Information on floods in the Susitna River during the summer of 1971 is given by Lamke (1972). The data herein were collected as part of a study and report on general scour at bridge crossings and local scour at bridge piers at sites in south-central and interior Alaska during 1965-72. The purpose of the study was to collect scour data at bridge sites and compare the results with existing laboratory data, field data, and predicted values from selected scour formulas. The report includes a detailed description of the physical setting, hydraulic characteristics, and channel geometry at low and high flows to assist the reader in developing background knowledge on the scour phenomenon in various situations.

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For the study, all indications of scour were considered to be related to either channel contraction or localized flow conditions at piers and abutments (local scour). All of the scour conditions probably occurred when there was significant bedload transport throughout the streams and Froude Numbers were less than 1, because scour at high flows was followed by fill. Information was obtained from floods greater than or equal to the mean annual flood. Soundings to determine cross-sectional profiles, longitudinal profiles, and scour-hole depths were generally obtained with a Raytheon Model DE119 D recording fathometer. Transducers used with the fathometer produced a 8-degree beam width. Equipment used to make soundings, measure velocities, and make discharge measurements were standard USGS equipment as described by Buchanan and Somers (1969). This equipment consists of the "B" reel, Price Type AA current meters, and sounding weights ranging from 50 to 100 lbs. Streambed-material samples were collected using samplers appropriate to stream velocities. Streambeds of sand and small gravel were sampled using a US-BM54. A locally constructed drag sampler was used in conjunction with a 100-lb sounding weight to sample bed material in gravel and cobble streams. Water-surface elevations were measured using standard surveying instruments and techniques. River stages were recorded by automatic recorders or by measuring down from a fixed point using a wire-weight gage. Photographs and surveys were used to aid in interpreting channel patterns, variations in channel cross-sectional shapes, and velocity distributions.

Elevation Reference

Datum:	Gage
Dacum	Gaye

MSL (ft): 208.35

Description of Reference Elevation:

Elevations given for pier definitions are to mean sea level. Elevations given for all stage data (including the hydrograph) and channel cross-section coordinates are given with reference to the gage datum, 208.35 ft (63.50 m)

Stream Data

Drainage Area (sq mi):	11500	Floodplain Width:	Unknown
Slope in Vicinity(ft/ft):	0.0004	Natural Levees:	Unknown
Flow Impact:	Right	Apparent Incision:	Unknown
Channel Evolution	Unknown	Channel Boundary:	Semi-alluvial
Armoring:	Unknown	Banks Tree Cover:	High
Debris Frequency:	Unknown	Sinuosity:	Unknown
Debris Effect:	Unknown	Braiding:	Generally
Stream Size:	Wide	Anabranching:	Generally
Flow Habit:	Perennial	Bars:	Wide

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Bed Material:	Cobbles	Stream Width Variability:	Wider
Valley Setting:	Unknown		

Roughness Data

Manning's n Values

Left Overbank Channel Right Overbank

High:

Typical

Low:

Bed Material

Measurement Number	Yr	Мо	Dy	Sampler	D95 (mm)	D84 (mm)	D50 (mm)		SP	Shape	Cohesion	
1	1971	7	2	Drag	96	85	70	58	2.65		Unknown	
2	1972	7	3	PhotoZeis s	128	105	77	56	2.65		Unknown	

Bed Material Comments

Measurement No: 1

Only the D90=90 and D50=70 were reported with the data. The D95, D84, and D16 were computed from the provided data. The D84 was interpolated from the D90 and D50 using a log-probability interpolation. Sigma was computed as D84/D50. D95 and D16 were computed from the equation D50 * Sigma^(standard normal deviate of 95 or 16).

Measurement No: 2

Only the D90=115 and D50=77 were reported with the data. The D95, D84, and D16 were computed from the provided data. The D84 was interpolated from the D90 and D50 using a log-probability interpolation. Sigma was computed as D84/D50. D95 and D16 were computed from the equation D50 * Sigma^(standard normal deviate of 95 or 16).

Bridge Data

Structure No:	254
Length(ft):	1072
Width(ft):	
Number of Spans:	5

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Vertical Configuration: Horizontal

Low Chord Elev (ft): 278

Upper Chord Elev (ft): 300

Overtopping Elev (ft):

Skew (degrees): 0

Guide Banks: None

Waterway Classification: Main

Year Built:

Avg Daily Traffic:

Plans on File: Yes

Parallel Bridges No

Upstream/Downstream: N/A

Continuous Abutment: No

Distance Between Centerlines:

Distance Between Pier Faces:

Bridge Description:

This bridge is 1072 ft long and is supported by four piers spaced 250 feet apart.

Abutment Data

Left Station: 0 Right Station: 0 Left Skew (deg): 0 Right Skew (deg) 0 Left Abutment Length (ft): Right Abutment Length (ft) Left Abutment to Channel Bank (ft): Right Abutment to Channel Bank (ft):

Right Abutment Protection	
Contracted Opening Type:	Unknown
Embankment Skew (deg):	0
Embankment Slope (ft/ft):	
Abutment Slope (ft/ft)	
Wingwalls:	No
Wingwall Angle (deg):	0

Pier Data

Pier ID	Bridge Station(ft)	Alignment	Highway :	Station	PierType	# Of Piles	Pile Spacing(ft)
1	700	0	0		Single	0	
2	950	0	0		Single	0	
3	1200	0	0		Single	0	
4	1450	0	0		Single	0	
Pier ID	Pier Width(ft)	Pier Shape	Shape F	actor I	Length(ft)	Protection	Foundation
1	5	Sharp			20	Unknown	Piles
2	5	Sharp			20	Unknown	Piles
3	5	Sharp			20	Unknown	Piles
4	5	Sharp			20	Unknown	Piles
Pier ID	Top Elevation(ttom tion(ft)		or Pile idth(ft) (Cap Shape	Pile Tip Elevation(ft)
1	236		226		21	Square	191
2	235		225		21	Square	190
3	235		225		21	Square	200
4	235		225		21	Square	200
Pier D	escription						

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Pier ID

1

2

3

4

The upper portion of the pier (whose height varies) is 5 ft wide and 20 ft long as described. The lower 15 ft is a 7-ft-wide by 28-ft-long round-nose stem. It rests on a 21-ft by 33-ft footing and a 24-ft by 36-ft seal, which are both 5 ft high and are founded on piles.

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Pier S	<u>icour L</u>	Data						
Pier	ID 1	Date	Time	USOrDS				
1	7	/2/71	0:00	Upstream				
1	8 /	11/71	0:00	Upstream				
2	7	/2/71	0:00	Upstream				
2	8 /	11/71	0:00	Unknown				
3	7	/2/71	0:00	Upstream				
3	8 /	11/71	0:00	Unknown				
4	7	/2/71	0:00	Downstream	m			
4	8 /	11/71	0:00	Downstream	m			
Pier ID	Scour Depth	Accuracy (ft)	Side Slope (ft/ft)	-		Apprch Depth(ft)	Effective Pier Width	Skew to Flow(deg)
1	2.5	0.5			6.5	19	5	0

10

8.5

17.5

13.5

Diar Scour Data

0.5

0.5

2

2.5

1

2

0

0

5

5

2	2	1			9.5			5	0
3 3	2 2	0.5 1			7 11.5	11 5 17		5 5	0
3 4	5	_ 0.5			5	13.		5	0
4	5	1			9.5			5	0
PierID	Sediment Transpoi		Bed terial	BedForm	Trough (ft)	Crest	Sigma	Debris Effect:	
1	Clear-wat	ter Non	-cohesive	Dune			1.2	Insigni	Eicant
1	Clear-wat	cer Non	-cohesive	Unknown			1.2	Insigni	ficant
2	Clear-wat	er Non	-cohesive	Dune			1.2	Insigni	Eicant
2	Clear-wat	er Non	-cohesive	Unknown			1.2	Insigni	Eicant
3	Clear-wat	er Non	-cohesive	Dune			1.2	Insigni	Eicant
3	Clear-wat	er Non	-cohesive	Unknown			1.2	Insigni	Eicant
4	Clear-wat	er Non	-cohesive	Dune			1.2	Substar	ntial
4	Clear-wat	cer Non	-cohesive	Unknown			1.2	Substar	ntial
Pier	ID 1	D95 (mm)	D84 (n	mm) D50	(mm)	D16 (m	nm.)		
1		96	85		70	58			
1		96	85		70	58			
2		96	85		70	58			
2		96	85		70	58			
3		96	85		70	58			
3		96	85		70	58			
4		96	85		70	58			
4		96	85		70	58			
Pier :	Scour Co	omments	5						
Pier II	b 1		Time:	0:00		US/DS:	Upst	cream	
Pier II	b 1		Time:	0:00		US/DS:	Upst	ream	
For the	is date,	this is	the only p	ier where	scour c	ould be a	ctuall.	y measure	d.

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Pier ID	2	Time:	0:00	US/DS:	Upstream					
Pier ID	2	Time:	0:00	US/DS:	Unknown					
This scou	ır measurement wa	s estim	ated from partial :	fathometer	trace.					
Pier ID	3	Time:	0:00	US/DS:	Upstream					
Pier ID	3	Time:	0:00	US/DS:	Unknown					
	ur measurement wa nd from results o		ated from soundings I in July 1971.	s on cross	sections at the					
Pier ID	4	Time:	0:00	US/DS:	Downstream					
be about	Submerged debris at the nose of the pier probably caused the scour depth to be about twice that expected if the debris had not been present. It also probably caused the scour to move further downstream.									

Pier ID 4 Time: 0:00 US/DS: Downstream

This scour measurement was estimated from soundings on cross sections at the bridge and from results obtained in July 1971. Submerged debris probably caued the scour depth to be twice that expected if debris had not been present and probably caused the scour to move downstream.

Abutment Scour

ContractionScour

Stage and Discharge Data

Pea	ak D	isch	arge	Flow			Peal	c Sta	age		Stage	Water	Return
year	mo	dy	hr mi	(cfs)	Qacc	year	mo	dy	hr	mi	(ft)	Temp (C)	Period(yr)
1971	8	11	(17100	0 none	1971	8	10		0	62	9	20
1971	7	2	(74600) none	1971	7	2		0	56.6	7.5	
1965	6	1	(80200) none	1965	6			0	56.2		
1971	5	27	(37400) none	1971	5	27		0	52.9		

Hydrograph

nyarograph								
Hydrograph Number	Year	Month	Day	Hr	Min	Sec	Stage(ft)	Discharge (cfs)
1	1971	6	1	0	0	0	52.7	
1	1971	6	5	0	0	0	53.2	
1	1971	6	8	0	0	0	57.1	
1	1971	6	11	0	0	0	57	
1	1971	6	13	0	0	0	59	
1	1971	б	19	0	0	0	55	
1	1971	б	25	0	0	0	58.2	
1	1971	б	28	0	0	0	57.6	
1	1971	б	30	0	0	0	58.1	
1	1971	7	5	0	0	0	55	
1	1971	7	12	0	0	0	55.9	
1	1971	7	15	0	0	0	57.8	
1	1971	7	20	0	0	0	56.2	
1	1971	7	25	0	0	0	53.2	
1	1971	7	28	0	0	0	54.2	
1	1971	7	30	0	0	0	54	

1	1971	8	4	0	0	0	56
1	1971	8	б	0	0	0	54.8
1	1971	8	10	0	0	0	62
1	1971	8	18	0	0	0	55
1	1971	8	22	0	0	0	55.7
1	1971	8	23	0	0	0	56
1	1971	8	25	0	0	0	55.4
1	1971	8	29	0	0	0	53.5
1	1971	9	14	0	0	0	52
1	1971	9	18	0	0	0	51.3
1	1971	9	22	0	0	0	51.5
1	1971	9	24	0	0	0	51.2
1	1971	9	25	0	0	0	50.8
1	1971	9	31	0	0	0	50

Supporting Files